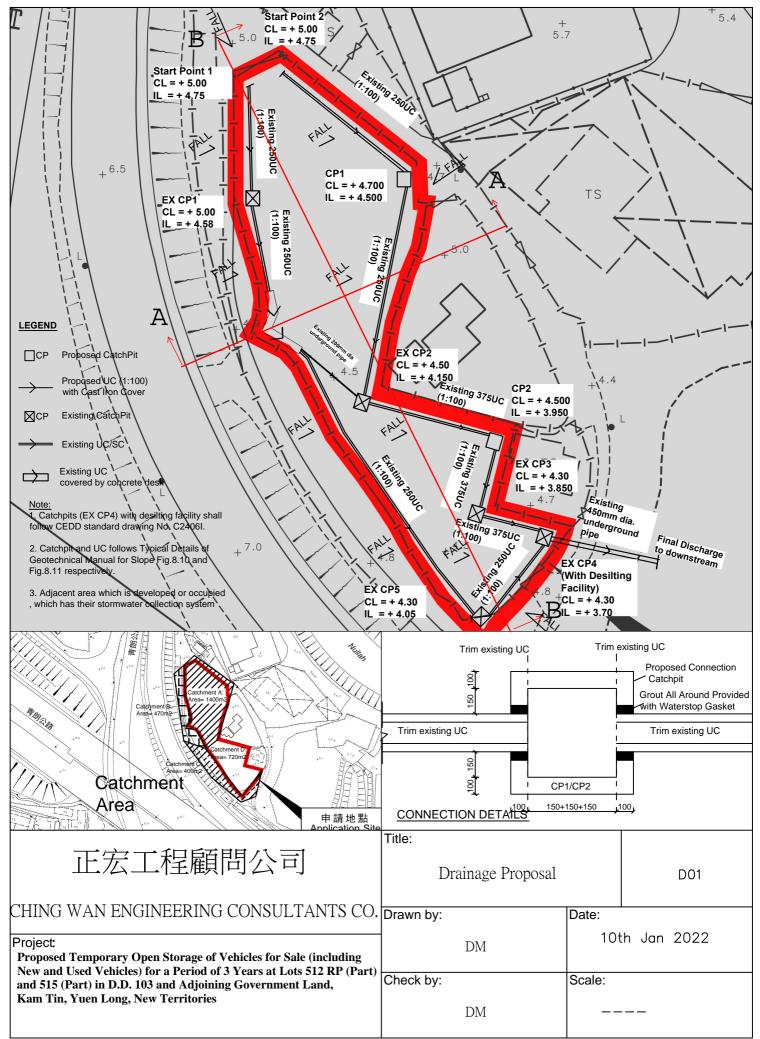
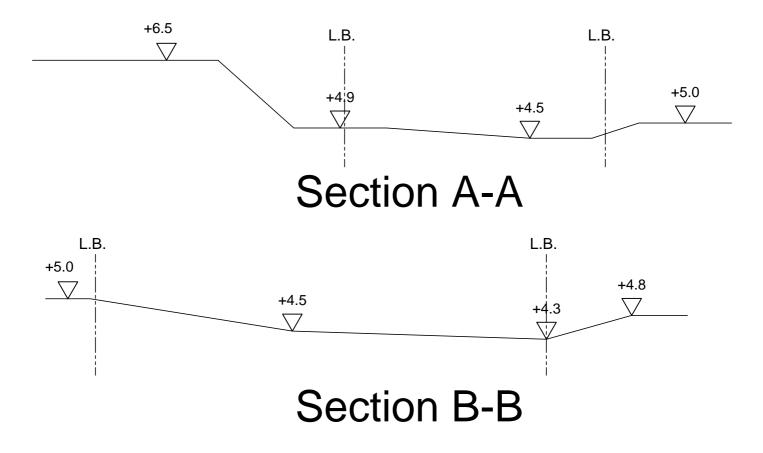
Appendix II





Company: Project :											
Date:	9/1/2022										
Calculation for channels:											
Catchment Area of site											
Catchment Area A	=	1100 0.0011	m^2 km^2								
Peak runoff in m^3/s	= = =	0.278 0.072628 4358	x m^3/s liter/mir	0.95 1	Х	250	mm/hr	x 0.0011	km^2		
According to (Figure 8.7 - C For gradient 1:100, existing				nnels),							
Catchment Area B+C	=	470 0.00087	m^2 km^2	+	400	m^2					
Peak runoff in m ³ /s	= = =	0.278 0.057442 3447	x m^3/s liter/mir	0.95 1	Х	250	mm/hr	x 0.00087	km^2		
According to (Figure 8.7 - C For gradient 1:100, existing				nnels),							
Catchment Area A+B+C+D	= =	1100 0.00269	m^2 km^2	+	470	m^2	+	400	m^2 +	720	m^2
Peak runoff in m ³ /s	= = =	0.278 0.177607 10656	x m^3/s liter/mir	0.95 1	Х	250	mm/hr	x 0.00269	km^2		

According to (Figure 8.7 - Chart for the Rapid Design of Channels), For gradient 1:100, existing 375UC will be suitable.

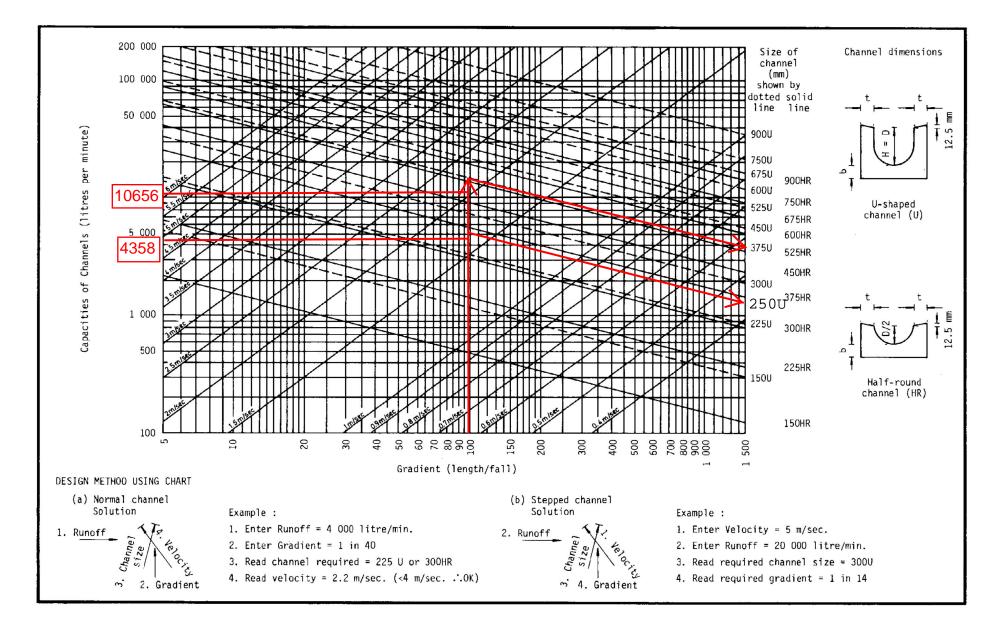


Figure 8.7 - Chart for the Rapid Design of Channels

Check existing 450mm dia. Pipes (1:100) by Colebrook-White Equation

$$V = -\sqrt{(8gDs)}\log(\frac{ks}{3.7D} + \frac{2.51v}{D\sqrt{(2gDs)}})$$

where	٠	
WIICIC		

where .									
V	=			mean vel	ocity (m/s)				
g	=	9.81	m/s2	gravitatio	onal acceleration (m/s2)				
D	=	0.45	m	internal p	oipe diameter (m)				
ks	=	0.00015	m	hydraulic	pipeline roughness (m)			(Table 5, from DSD S	Sewerage Manual, concrete pipe)
V	=	1.14E-06	m2/s	kinemati	c viscosity of fluid (m2/s	.)			
S	=	0.005		hydraulic	gradient				
Area A	=	0.159043	m2						
Therefore, design V of pipe	=	1.6470	m/s	>	Design velocity from	=	0.1776	m3/s /	0.159043128
					catchment area	=	1.116724	m/s	===>O.K.

Therefore, 450mm dia. pipe (1:100) will be adopted for connection bewteen site and final discharge

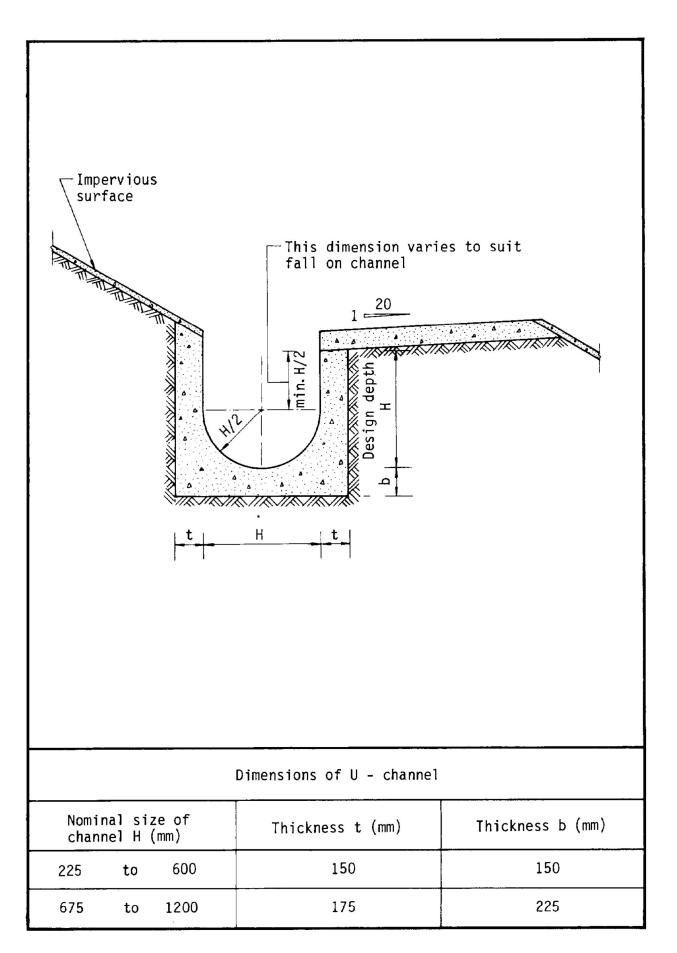


Figure 8.11 - Typical U-channel Details

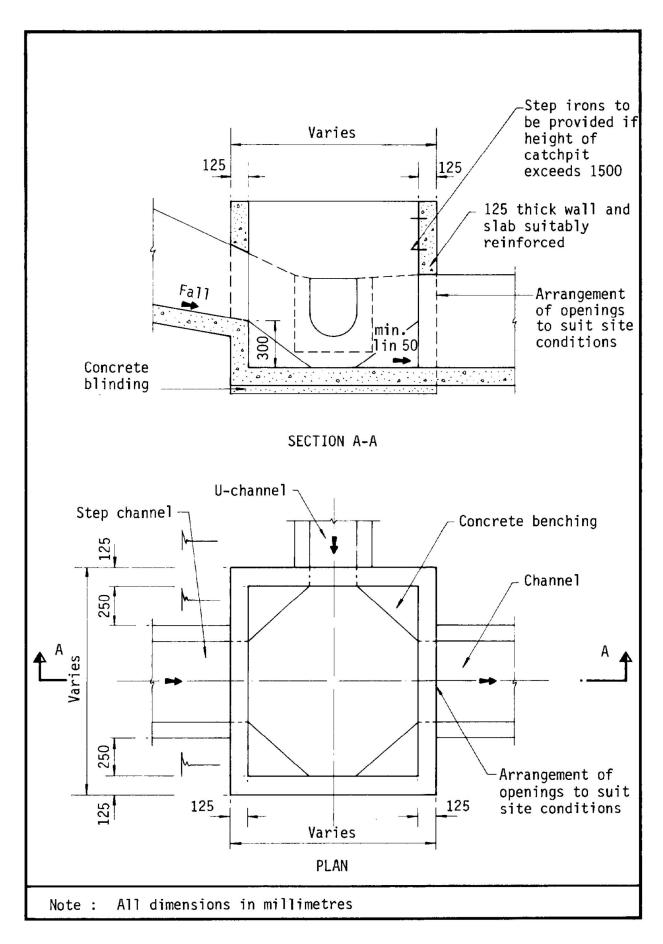
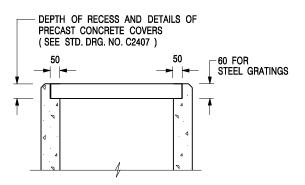


Figure 8.10 - Typical Details of Catchpits



ALTERNATIVE TOP SECTION FOR PRECAST CONCRETE COVERS / GRATINGS

NOTES:

- 1. ALL DIMENSIONS ARE IN MILLIMETRES.
- 2. ALL CONCRETE SHALL BE GRADE 20 /20.
- 3. CONCRETE SURFACE FINISH SHALL BE CLASS U2 OR F2 AS APPROPRIATE.
- 4. FOR DETAILS OF JOINT, REFER TO STD. DRG. NO. C2413.
- 5. CONCRETE TO BE COLOURED AS SPECIFIED.
- UNLESS REQUESTED BY THE MAINTENANCE PARTY AND AS DIRECTED BY THE ENGINEER, CATCHPIT WITH TRAP IS NORMALLY NOT PREFERRED DUE TO PONDING PROBLEM.
- 7. UPON THE REQUEST FROM MAINTENANCE PARTY, DRAIN PIPES AT CATCHPIT BASE CAN BE USED BUT THIS IS FOR CATCHPITS LOCATED AT SLOPE TOE ONLY AND AS DIRECTED BY THE ENGINEER.
- 8. FOR CATCHPITS CONSTRUCTED ON OR ADJACENT TO A FOOTPATH, STEEL GRATINGS (SEE DETAIL 'A' ON STD. DRG. NO. C2405) OR CONCRETE COVERS (SEE STD. DRG. NO. C2407) SHALL BE PROVIDED AS DIRECTED BY THE ENGINEER.
- 9. IF INSTRUCTED BY THE ENGINEER, HANDRAILING (SEE DETAIL 'G' ON STD. DRG. NO. C2405; EXCEPT ON THE UPSLOPE SIDE) IN LIEU OF STEEL GRATINGS OR CONCRETE COVERS CAN BE ACCEPTED AS AN ALTERNATIVE SAFETY MEASURE FOR CATCHPITS NOT ON A FOOTPATH NOR ADJACENT TO IT. TOP OF THE HANDRAILING SHALL BE 1 000 mm MIN. MEASURED FROM THE ADJACENT GROUND LEVEL.
- 10. MINIMUM INTERNAL CATCHPIT WIDTH SHALL BE 1 000 mm FOR CATCHPITS WITH A HEIGHT EXCEEDING 1 000 mm MEASURED FROM THE INVERT LEVEL TO THE ADJACENT GROUND LEVEL. AND, STEP IRONS (SEE DSD STD. DRG. NO. DS1043) AT 300 ¢ STAGGERED SHALL BE PROVIDED. THICKNESS OF CATCHPIT WALL FOR INSTALLATION OF STEP IRONS SHALL BE INCREASED TO 150 mm.
- 11. FOR RETROFITTING AN EXISTING CATCHPIT WITH STEEL GRATING, SEE DETAIL 'F' ON STD. DRG. NO. C2405.
- 12. SUBJECT TO THE APPROVAL OF THE ENGINEER, OTHER MATERIALS CAN ALSO BE USED AS COVERS / GRATINGS.

	– FORMER DRG.	. NO. C2406J. Original Signed 03.2015					
	REF. F	REVISION SIGNATURE DATE					
CATCHPIT WITH TRAP	CEDD CI DEV	CIVIL ENGINEERING AND DEVELOPMENT DEPARTMENT					
(SHEET 2 OF 2)	SCALE 1:20	DRAWING NO.					
(SIILLI Z OI Z)	DATE JAN 19	ogi C2406 /2					
卓越工程 建設香港	We Enginee	We Engineer Hong Kong's Development					

